



TEST REPORT

Rendered to:

EASTERN WHOLESALE FENCE COMPANY, INC.

For:

8 ft by 42 in *Eastern Ornamental Railing*Aluminum Guardrail System

Report No.: F5524.01-119-19

Report Date:

03/31/16

Test Record Retention Date:

02/26/20





TEST REPORT

F5524.01-119-19 March 31, 2016

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TEST REPORT

Rendered to:

EASTERN WHOLESALE FENCE COMPANY, INC. 301 Scott Avenue Calverton, New York 11933

Report No.: F5524.01-119-19

Test Date: 02/26/16 Report Date: 03/31/16

Test Record Retention Date: 02/26/20

1.0 General Information

1.1 Product

8 ft by 42 in Eastern Ornamental Railing - Aluminum Guardrail System

1.2 Project Description

Architectural Testing, Inc., an Intertek company ("Intertek-ATI"), was contracted by Eastern Wholesale Fence Company, Inc. to perform structural testing on their 8 ft by 42 in *Eastern Ornamental Railing* aluminum guardrail system. The purpose of the testing is preliminary code compliance evaluation in accordance with the following criteria:

ICC-ES™ AC273 (March 1, 2008 - Editorial Revised January 2012), Acceptance Criteria for Handrails and Guards

ICC-ES™ AC273-08 was developed by the ICC Evaluation Service, Inc. (ICC-ES™) as acceptance criteria to evaluate compliance with the following building codes:

2012 International Building Code®, International Code Council

2012 International Residential Code®, International Code Council

1.3 Limitations

All tests performed were to evaluate structural performance of the railing assembly to carry and transfer imposed loads to the supports (posts). The test specimen evaluated included the pickets, rails, rail brackets, posts, and attachment to the supporting structure. Anchorage of support posts to the supporting structure is not included in the scope of this testing and would need to be evaluated separately.





1.4 Qualifications

Intertek-ATI has demonstrated compliance with ISO/IEC International Standard 17025 and is consequently accredited as a Testing Laboratory (TL-144) by International Accreditation Service, Inc. (IAS).

1.5 Product Description

The 8 ft by 42 in *Eastern Ornamental Railing* aluminum guardrail system is comprised of aluminum rails, pickets, and posts produced by an extrusion process. Drawings are included in Appendix A to verify the overall dimensions and other pertinent information of the tested product, its components, and any constructed assemblies.

1.6 Product Sampling

All samples utilized for testing reported herein were provided to Intertek-ATI by Eastern Wholesale Fence Company, Inc. and were not independently sampled and selected.

1.7 Witnessing

Mr. Philip Bryan was present on 02/26/16 to witness structural performance testing of assembled railing systems conducted and reported herein.

1.8 Conditions of Testing

Unless otherwise indicated, all testing reported herein was conducted in a laboratory set to maintain temperature in the range of 68 ± 4 °F and humidity in the range of 50 ± 5 % RH.

2.0 Referenced Standard

ASTM D1761-12, Standard Test Methods for Mechanical Fasteners in Wood

3.0 Assembly Fastener Testing

Re: ICC-ES™ AC273 - Section 4.2.7

3.1 General

The purpose of this testing was to simulate a 90 degree bracket loading condition, which addresses a situation when the guardrail system is to be installed with the top rails in a corner condition.





3.2 Test Specimens

Short sections of the top rail were attached in accordance with Eastern Wholesale Fence Company, Inc.'s installation instructions to short sections of posts. Specimens were assembled by an Intertek-ATI technician. Rail brackets were secured to the post and to the rail as described in Section 4.4 Fastening Schedule.

3.3 Test Setup

The testing machine was fitted with the post sections at the top and bottom to accommodate anchorage of the rail and brackets. The top post section was attached to the test machine's crosshead with a swivel mechanism, and the bottom post section was attached rigidly to the base of the test machine. Three specimens were tested in this manner with each of the three specimens including two connections for a total of six connections. See photograph in Appendix B for test setup.

3.4 Test Procedure

Testing was performed in accordance with ASTM D 1761 and by using a computer-monitored and -controlled SATEC Unidrive, Model MII 50 UD Universal Testing Machine. Tests were run at a crosshead speed of 0.05 in/min, and each specimen was tested in tension to its ultimate load capacity. Testing was conducted on 02/29/16.

3.5 Test Results

Sample No.	Ultimate Load (lb)	Deviation From Average	Mode of Failure
1	1736	+9.2%	
2	1512	-4.9%	Bracket to rail fastener
3	1522	-4.2%	shear failure
Average	1590		
Allowable Capacity ¹	530	≥ 200 lb OK	

¹ Average ultimate load divided by a factor of safety of three (3.0)

3.6 Summary and Conclusions

The maximum design load rating required for guardrail systems for use in IRC - One- and Two-Family Dwellings and for rail lengths up to and including 8 ft for use in IBC - All Use Groups is 200 lb. Therefore, fasteners / connectors reported herein meet the performance requirements of ICC-ES™ AC273 for use in corner conditions.





4.0 Structural Performance Testing of Assembled Railing Systems

Re: ICC-ES™ AC273 - Section 4.2.1

4.1 General

Railing assemblies were tested in a self-contained structural frame designed to accommodate anchorage of a rail assembly and application of the required test loads. The specimen was loaded using an electric winch mounted to a rigid steel test frame. High strength steel cables, nylon straps, and load distribution beams were used to impose test loads on the specimen. Applied load was measured using an electronic load cell located in-line with the loading system. Deflections were measured to the nearest 0.01 in using electronic linear displacement transducers.

4.2 Railing Assembly Description

The Eastern Ornamental Railing aluminum guardrail systems consisted of aluminum top and bottom rails with spaced pickets between the rail members. The railing systems had an overall top rail length (inside of post to inside of post) of 93-1/8 in with an overall rail height (top of top rail to bottom of bottom rail) of 39-1/8 in. Top and bottom rails attached to aluminum post mounts via cast aluminum collar and socket brackets respectively. See Section 4.4 Fastening Schedule for connection details. A support block was located at the midspan of the bottom rail and was attached according to Section 4.4 Fastening Schedule. See drawings in Appendix A and photographs in Appendix B for additional details.

4.3 Series / Model

The test specimen components were supplied by Eastern Wholesale Fence Company, Inc. and were assembled by a representative of Intertek-ATI.

<u>Top Rail</u>: 2-1/8 in wide by 2-1/4 in high by 0.080 in wall 6061-T6 aluminum contoured (breadloaf profile) rail with internal 1-1/8 in wide by 7/8 in high by 0.035/0.090 in wall inverted U-shaped vinyl baluster retainer and 7/8 in wide by 7/32 in high by 0.050 in wall 6061-T6 aluminum baluster spacer

Bottom Rail: 1-1/4 in wide by 1-1/2 in high by 0.050 in wall 6061-T6 aluminum inverted U-shaped profile rail with internal 1-1/8 in wide by 7/8 in high by 0.035/0.090 in wall U-shaped vinyl baluster retainer

<u>Brackets</u>: - Top Rail – 2-3/8 in wide by 2-1/8 in high by 1-1/2 in deep ADC-12 cast aluminum socket bracket

 Bottom Rail – 1-1/2 in wide by 2-3/8 in high by 1-1/2 in deep ADC-12 cast aluminum socket bracket

Pickets: 3/4 in square by 0.040 in wall 6061-T6 aluminum pickets





4.3 Series / Model (Continued)

<u>Support Block</u>: Three piece section consisting of a 7/8 in long section of picket secured to the bottom rail as described in Section 4.4 Fastening Schedule, a 1 in square by 0.050 in wall 6061-T6 aluminum section cut to length and an ADC-12 cast aluminum socket bracket secured to the deck surface as described in Section 4.4 Fastening Schedule.

- Post: 2-1/2 in square by 0.110 in wall, 6061-T6 aluminum post with raceway channels at each interior corner attached to 4-1/4 in square by 1/2 in thick steel base plate with four 3/8 in by 2-3/4 in (17 TPI, 0.320 in minor diameter) hex head, carbon steel bolts; the base plate contained four 1/2 in diameter through holes (3-3/8 in oncenter; 7/16 in edge of plate to center of hole), four 7/16 in diameter through holes with 13/16 in diameter by 1/4 in deep counter bores (2 in on-center; 1-1/8 in edge of plate to center of hole) and one 7/16 in diameter through hole directly in the middle the base plate was attached to the surface of a rigid steel test surface (simulated concrete) as described in Section 5.4 Fastening Schedule for the post mount test only.
 - 3 in square by 0.120 in wall, 6061-T6 aluminum post with raceway channels at each interior corner attached to 4-3/4 in square by 1/2 in thick steel base plate with four 3/8 in by 2-3/4 in (17 TPI, 0.320 in minor diameter) hex head, carbon steel bolts; the base plate contained four 1/2 in diameter through holes (3-7/8 in on-center; 7/16 in edge of plate to center of hole), four 7/16 in diameter through holes with 7/8 in diameter by 1/4 in deep counter bores (2-1/2 in on-center; 1-1/8 in edge of plate to center of hole) and one 7/16 in diameter through hole directly in the middle the base plate was attached to the surface of a rigid steel test surface (simulated concrete) as described in Section 5.4 Fastening Schedule for the post mount test only.

See drawings in Appendix A and photographs in Appendix B for additional details.





4.4 Fastening Schedule

Connection	Fastener		
Top Rail Bracket to Post	Three #10-12 by 1" (0.132 in minor diameter) pan-head,		
TOP Nati Bracket to Post	Philips drive, self-drilling, stainless steel screws		
Bottom Rail Bracket to Post	Two #10-12 by 1" (0.132 in minor diameter) pan-head,		
Bottom Nam Bracket to Post	Philips drive, self-drilling, stainless steel screws		
Top Rail Bracket to Rail	Two #8-18 by 5/8" (0.110 in minor diameter) pan-head,		
TOP Nass Blacket to Nati	Philips drive, self-drilling, stainless steel screws		
Bottom Rail Bracket to Rail	Slip fit – no mechanical connections		
Baluster to Top and Bottom Rail	Slip fit – no mechanical connections		
Support Block Picket Section	One #8-18 by 1/2" (0.112 in minor diameter) pan-head,		
to Bottom Rail	Philips drive, self-drilling, stainless steel screw		
Support Block Bracket to Deck	Two #10-12 by 1" (0.132 in minor diameter) pan-head,		
Surface	Philips drive, self-drilling, stainless steel screws		
Post Mount to Substructure	Four 3/8 in Grade 8 hex-head bolts with washer		

4.5 Test Setup

The railing assembly was installed and tested as a single railing section by directly securing the aluminum post to a rigid test frame (stanchions) (fully assembled guardrail testing) or by directly securing (surface-mounting; simulated concrete) the base of the post mounts to a rigid steel test frame (post mount testing). The railing was assembled by an Intertek-ATI technician. Transducers mounted to an independent reference frame were located to record movement of reference points on the railing system components (ends and mid-point) to determine net component deflections. See photographs in Appendix B for test setups.





4.6 Test Procedure

Testing and evaluation was performed in accordance with Section 4.2.1 of ICC-ES™ AC273. The test specimen was inspected prior to testing to verify size and general condition of the materials, assembly, and installation. No potentially compromising defects were observed. One specimen was used for all load tests which were performed in the order reported. Each design load test was performed using the following procedure:

- 1. Zeroed transducers and load cell at zero load;
- 2. Increased load to specified test load in no less than ten seconds; and
- 3. Held test load for no less than one minute.

4.7 Test Results

Unless otherwise noted, all loads and displacement measurements were normal to the rail (horizontal). The test results apply only to the railing assembly between supports and anchorage to the support.

Key to Test Results Tables:

Load Level: Target test load

<u>Test Load</u>: Actual applied load at the designated load level (target). Where more than one value is reported, the test load was the range (min. - max.) that was held during the time indicated in the test.

<u>Elapsed Time (E.T.)</u>: The amount of time into the test with zero established at the beginning of the loading procedure. Where more than one value is reported, the time was the range (start-end) that the designated load level was reached and sustained.





93-1/8 in by 42 in Eastern Ornamental Railing Aluminum Guardrail IBC – All Use Groups / ICC-ES™ AC273 Specimen No. 1 of 2

Design	Test No. 1 - Test Date: 02/26/16 Design Load: 50 lb / 1 Square ft of In-Fill at Center of Two Pickets							
Load Level	Test Load (lb)	E.T. (min:sec)	Result					
125 lb (2.50 x D.L.)	126 - 130	00:29 - 01:31	Sustained load equal to or greater than 125 lb for one full minute without failure					

Design	Test No. 2 - Test Date: 02/26/16 Design Load: 50 lb / 1 Square ft of In-Fill at Bottom of Two Pickets							
Load Level	Load Level Test Load (lb) E.T. (min:sec) Result							
125 lb (2.50 x D.L.)	124 - 131 ¹	00:24 - 01:27	Sustained load equal to or greater than 125 lb for one full minute without failure					

 $^{^{1}}$ The test load fell below the target load level for a total of 1 second while maintaining load.

Test No. 3 - Test Date: $02/26/16$ Design Load: $50 \text{ plf x } (93-1/8 \text{ in } \div 12 \text{ in/ft}) = 388 \text{ lb Uniform Load}$ at 45° from Horizontal on Top Rail ¹							
Load Level	Test Load (lb)	E.T. (min:sec)	Result				
970 lb (2.50 x D.L.)	972 - 981	01:19 - 02:20	Sustained load equal to or greater than 970 lb for one full minute without failure				

¹ Uniform load was simulated with quarter point loading





Specimen No. 1 of 2 (Continued)

Test No. 4 - Test Date: 02/26/16 Design Load: 200 lb Concentrated Load at Mid-Span of Top Rail								
Load Level Test Load E.T. Displacement (in)								
	(lb)	(min:sec)	End	Mid	End	Net ¹		
200 lb (D.L.)	201	00:35	0.03	1.13	0.03	1.10		
500 lb (2.50 x D.L.)	503 - 512	01:01 - 02:04		0 lb for one	ad equal to full minute ure	~		

Deflection Evaluation:

Maximum rail deflection at 201 lb = 1.10 in on an 8 ft raif (93-1/8 in)

Limits per AC273²:
$$\left(\frac{h}{24} + \frac{l}{96}\right) = \left(\frac{36}{24} + \frac{93.125}{96}\right) = 2.47" > 1.10" : ok$$

and
$$\frac{h}{12} = \frac{42}{12} = 3.50^{\circ} > 1.10^{\circ} \therefore \text{ ok}$$

² Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements.

Design Load	Test No. 5 - Test Date: 02/26/16 Design Load: 200 lb Concentrated Load at Both Ends of Top Rail (Brackets)							
Load Level ¹	Load Level ¹ Test Load E.T. Result (lb) (min:sec)							
1000 lb (2.50 x D.L.) x 2	1000 - 1028	00:38 - 01:41	Each end withstood load equal to or greater than 500 lb for one full minute without failure.					

¹ Load was imposed on both ends of rail using a spreader beam; therefore, loads were doubled.

¹ Each end displacement was measured at the center of the support. Net displacement was the rail displacement relative to the supports.





Specimen No. 2 of 2

Design	Test No. 1 - Test Date: 02/26/16 Design Load: 50 lb / 1 Square ft of In-Fill at Center of Two Pickets							
Load Level	Test Load (lb)	E.T. (min:sec)	Result					
125 lb (2.50 x D.L.)	127 - 133	00:26 - 01:27	Sustained load equal to or greater than 125 lb for one full minute without failure					

Design	Test No. 2 - Test Date: 02/26/16 Design Load: 50 lb / 1 Square ft of In-Fill at Bottom of Two Pickets							
Load Level	Load Level Test Load (lb) E.T. (min:sec) Result							
125 lb (2.50 x D.L.)	128 - 139	00:22 - 01:25	Sustained load equal to or greater than 125 lb for one full minute without failure					

Design	Load: 50 plf x (93	3 - Test Date: 02/ -1/8 in ÷ 12 in/ft) n Horizontał on To	= 388 lb Uniform Load
Load Level	Test Load (lb)	E.T. (min:sec)	Result
970 lb (2.50 x D.L.)	973 - 987	00:49 - 01:58	Sustained load equal to or greater than 970 lb for one full minute without failure

¹ Uniform load was simulated with quarter point loading





Specimen No. 2 of 2 (Continued)

Test No. 4 - Test Date: 02/26/16 Design Load: 200 lb Concentrated Load at Mid-Span of Top Rail							
Load Level Test Load E.T. Displacement (in)							
Load Level	(lb)	(min:sec)	End	Mid	End	Net ²	
200 lb (D.L.)	200	00:31	0.02	1.12	0.03	1.10	
500 lb (2.50 x D.L.)	502 - 508	00:56 - 01:59	Result: Withstood load equal to or gr than 500 lb for one full minute with failure				

Deflection Evaluation:

Maximum rail deflection at 200 lb = 1.10 in on an 8 ft rail (93-1/8 in)

Limits per AC273 ²:
$$\left(\frac{h}{24} + \frac{1}{96}\right) = \left(\frac{36}{24} + \frac{93.125}{96}\right) = 2.47'' > 1.10'' \therefore ok$$

and $\frac{h}{12} = \frac{42}{12} = 3.50'' > 1.10'' \therefore ok$

² Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements.

Test No. 5 - Test Date: 02/26/16 Design Load: 200 lb Concentrated Load at Roth Ends of Ten Bail (Brackets)					
Design Load	Design Load: 200 lb Concentrated Load at Both Ends of Top Rail (Brackets)				
Load Level ¹	Test Load (lb)	E.T. (min:sec)	Result		
1000 lb (2.50 x D.L.) x 2	1002 - 1031	00:48 - 01:51	Each end withstood load equal to or greater than 500 lb for one full minute without failure.		

¹ Load was imposed on both ends of rail using a spreader beam; therefore, loads were doubled.

¹ Each end displacement was measured at the center of the support. Net displacement was the roil displacement relative to the supports.





Stand-Alone Post Mount Testing Simulated Concrete Application 2-1/2 in Post Mounts

Desig	Test No. 1 - Test Date: 03/04/16 Design Load: 200 lb Concentrated Load at Top of Post Mount (42 in High)				
Load Level Test Load E.T. Displacement (in)					
200 lb (D.L.)	200	00:35	0.54		
Ultimate Load:	822	01:08	Result: Failure of attachment bolt on underside of post mount (shear failure)		

Deflection Evaluation:

Maximum post deflection at 200 lb = 0.54 in

Limits per AC273 ¹:
$$\frac{h}{12} = \frac{36}{12} = 3.00" > 0.54"$$
 .. ok

Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements

Desig	Test No. 2 - Test Date: 03/04/16 Design Load: 200 lb Concentrated Load at Top of Post Mount (42 in High)				
Load Level Test Load E.T. Displacement (in)					
200 lb (D.L.)	202	00:26	0.57		
Ultimate Load:	783	01:02	Result: Post snapped off above attachment fasteners		

Deflection Evaluation:

Maximum post deflection at 202 lb = 0.57 in

Limits per AC273
1
: $\frac{h}{12} = \frac{36}{12} = 3.00^{\circ} > 0.57^{\circ}$ \therefore ok

Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements





Stand-Alone Post Mount Testing Simulated Concrete Application 3 in Post Mounts

Test No. 1 - Test Date: 03/04/16 Design Load: 200 lb Concentrated Load at Top of Post Mount (42 in High)				
Load Level Test Load E.T. Displacement (in)				
200 lb (D.L.)	203	00:18	0.46	
Ultimate Load:	1049	00:57	Result: Failure of attachment bolt on underside of post mount (shear failure)	

Deflection Evaluation:

Maximum post deflection at 203 lb = 0.46 in

Limits per AC273 ¹:
$$\frac{h}{12} = \frac{36}{12} = 3.0^{\circ} > 0.46^{\circ}$$
 \therefore ok

Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements

Test No. 2 - Test Date: 03/04/16 Design Load: 200 lb Concentrated Load at Top of Post Mount (42 in High)				
Load Level Test Load E.T. Displacement (in)				
200 lb (D.L.)	202	00:22	0.47	
Ultimate Load:	542	00:40	Result: Failure of attachment bolt on underside of post mount (shear failure)	

Deflection Evaluation:

Maximum post deflection at 202 lb = 0.47 in

Limits per AC273 1:
$$\frac{h}{12} = \frac{36}{12} = 3.0" > 0.47" : ok$$

Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements





Stand-Alone Post Mount Testing Simulated Concrete Application 3 in Post Mounts

(Continued)

Test No. 3 - Test Date: 03/08/16 Design Load: 200 lb Concentrated Load at Top of Post Mount (42 in High)				
Load Level Test Load E.T. Displacement (in)				
200 lb (D.L.)	203			
Ultimate Load:	1067	01:10	Result: Failure of attachment bolt on underside of post mount (shear failure)	

Deflection Evaluation:

Maximum post deflection at 203 lb = 0.48 in

Limits per AC273
$$\frac{1}{12} = \frac{36}{12} = 3.0^{\circ} > 0.48^{\circ} \therefore \text{ o k}$$

4.8 Summary and Conclusions

Preliminary evaluation concludes that the railing assembly reported herein meets the structural performance requirements of Section 4.2.1 of ICC-ES™ AC273 for use in Commercial Applications (IBC). In order to qualify the guardrail as AC273 compliant additional testing would need to be conducted of product sampled by an independent inspection agency.

Anchorage of support posts to the supporting structure is not included in the scope of this testing and would need to be evaluated separately.

Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements







5.0 Closing Statement

Intertek-ATI will service this report for the entire test record retention period. Test records that are retained such as detailed drawings, datasheets, representative samples of test specimens, or other pertinent project documentation will be retained by Intertek-ATI for the entire test record retention period.

Results obtained are tested values and were secured using the designated test methods. This report does not constitute certification of this product nor an opinion or endorsement by this laboratory. It is the exclusive property of the client so named herein and relates only to the specimens tested. This report may not be reproduced, except in full, without the written approval of Intertek-ATI.

For INTERTEK-ATI:

Digitally Signed by: Adam J. Schrum

Adam J. Schrum Lead Technician Digitally Signed by: Virgal Thomas Mickley, Jr.

V. Thomas Mickley, Jr., P.E. Senior Project Engineer

AJS:vtm/jas

Attachments (pages): This report is complete only when all attachments listed are included.

Appendix A - Drawings (15) Appendix B - Photographs (4)





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Revision Log

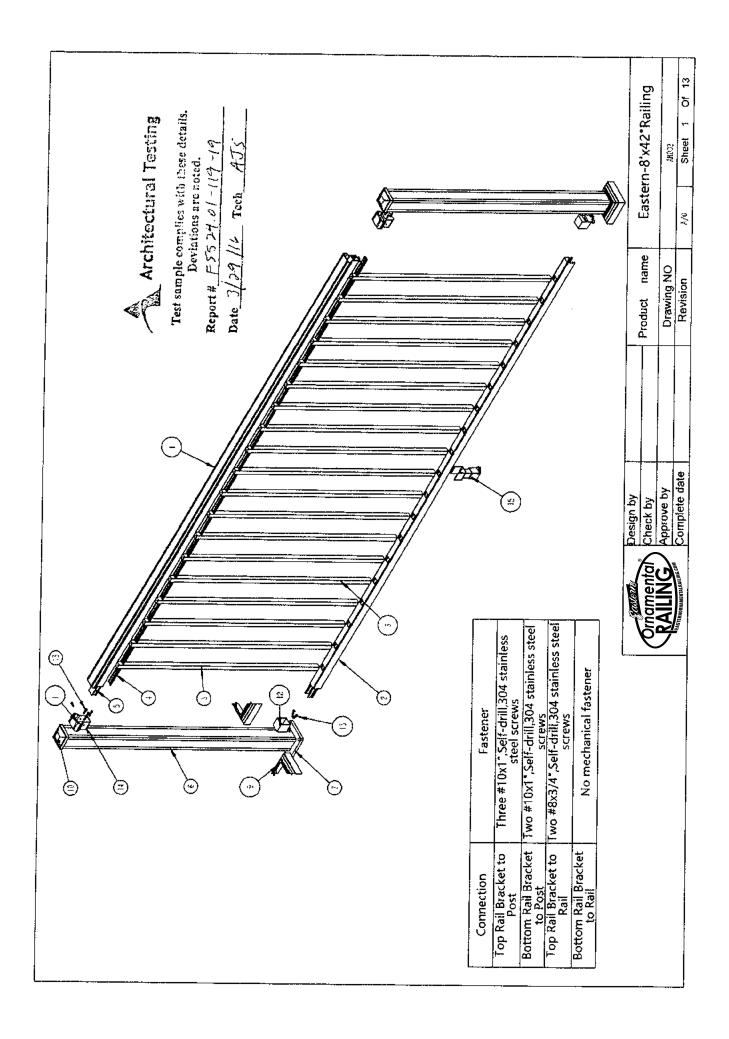
<u>Rev. #</u>	Date	Page(s)	Revision(s)
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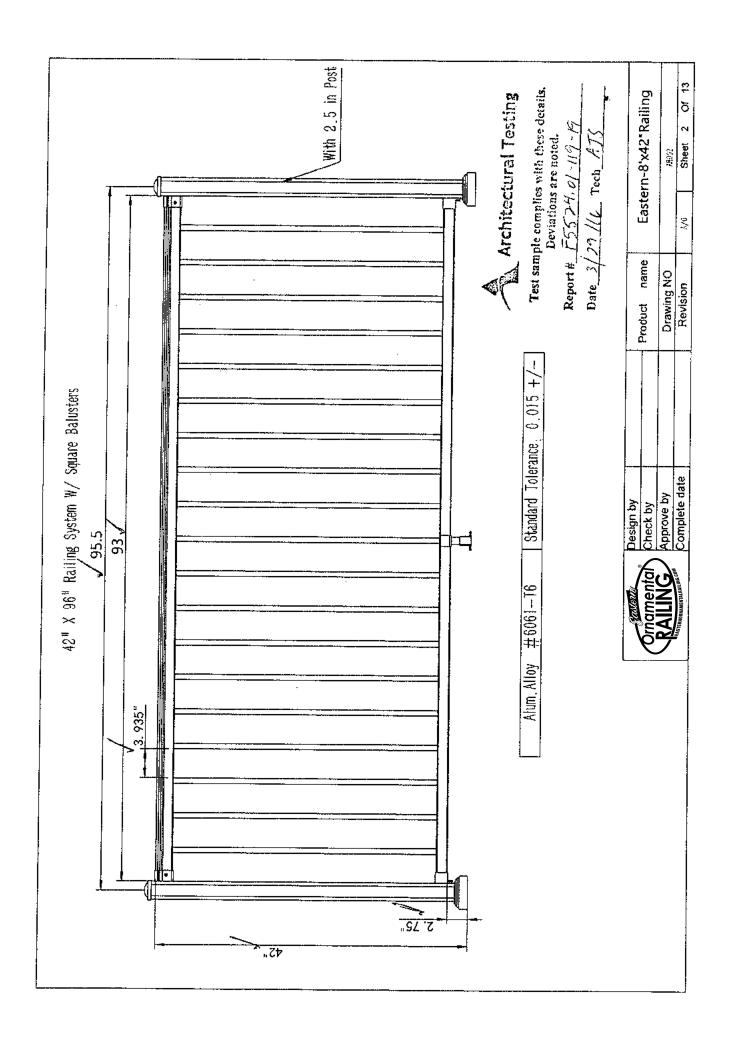


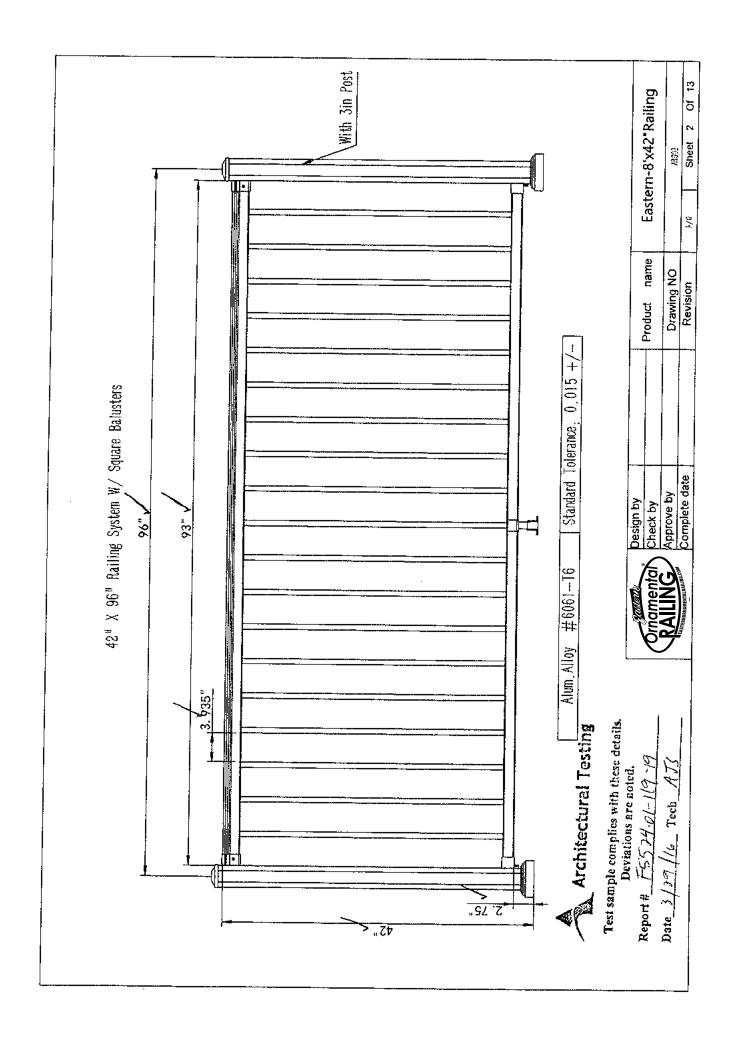


APPENDIX A

Drawings







Test sample complies with these details.

Report # 755.24.0/ 1/9-19

Date 1/29/12 Tech 473

	Parts List	ist	
S.N.	ltem	QTY	Material
-	top rail 2125	-	Aluminum 6061-T6
2	bottom rail 12515		Aluminum 6061-T6
m	3/4" picket	19	Aluminum 6061-T6
4	Spacer	20	Aluminum 6061-T6
ī2	vinyl insert		vinyl
9	2.5" post	2	Aluminum 6061-T6
7	base plate	2	#45 steel
∞	0.4x2 3/4 Bolt (base plate to post)	8	carbon steel Level 8.8
6	2.5"Post trim	2	ADC-12
10	2.5" post cap	2	ADC-12
=	top bracket	2	ADC-12
12	bottom bracket	2	ADC-12
13	10# 1"Screw (all brackets to post)	10	SS304
14	8# 3/4"Screw (top bracket to rail)	4	SS304
15	bottom rail support	1	Aluminum 6061-T6



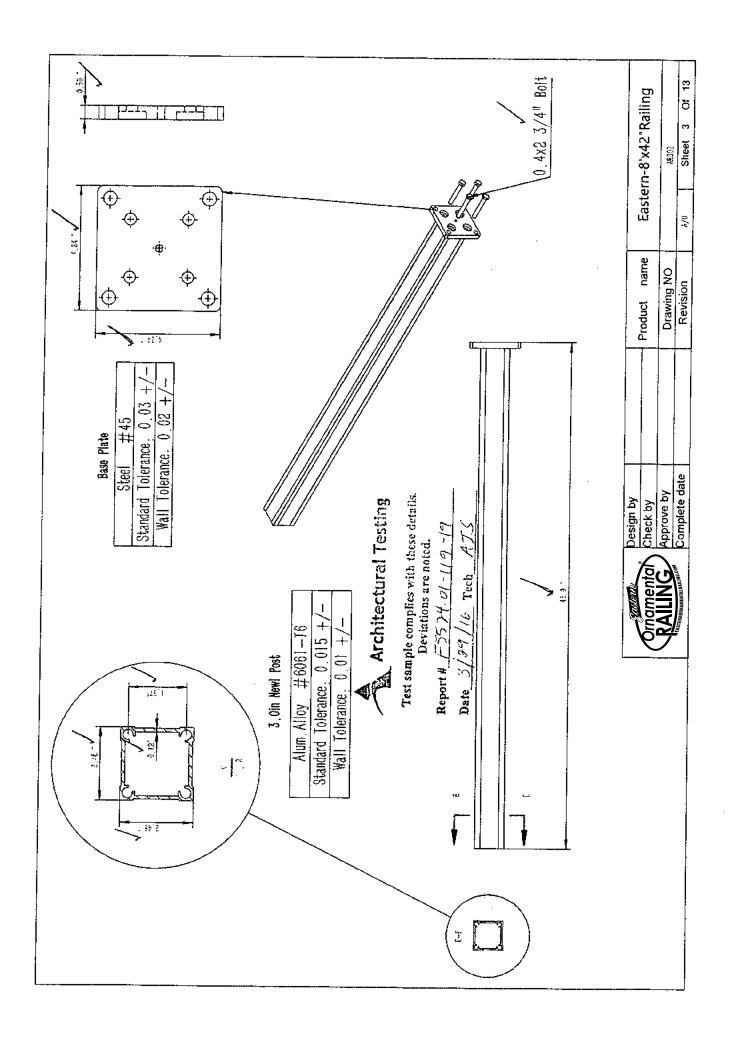
Test sample complies with these details.

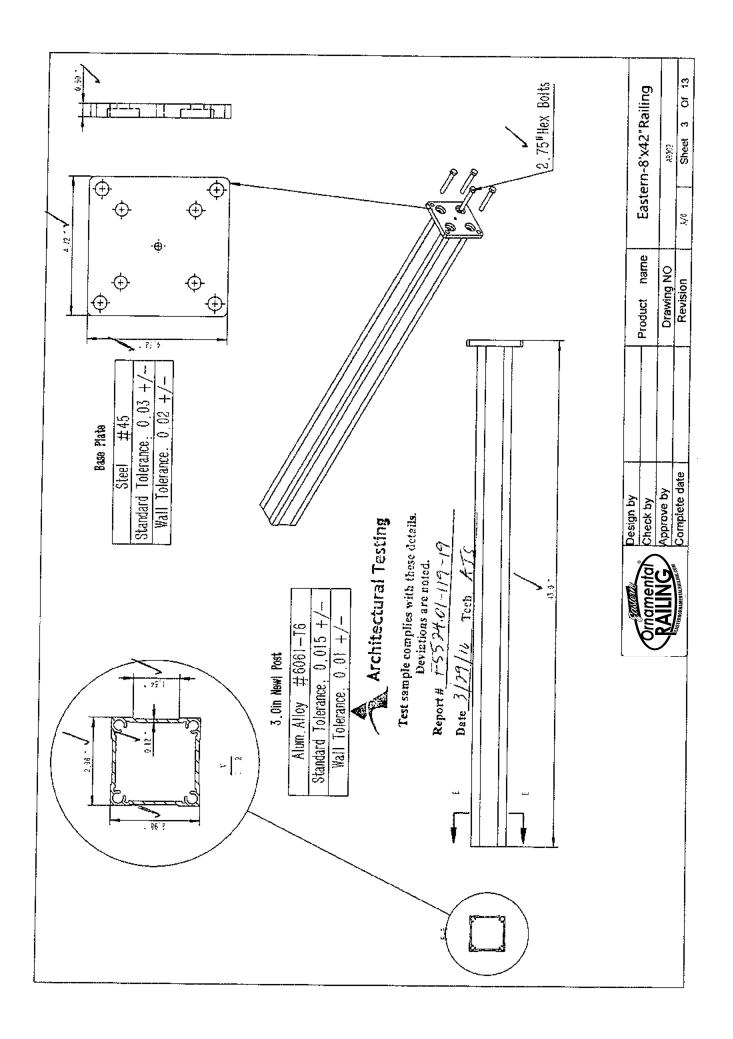
Deviations are noted.

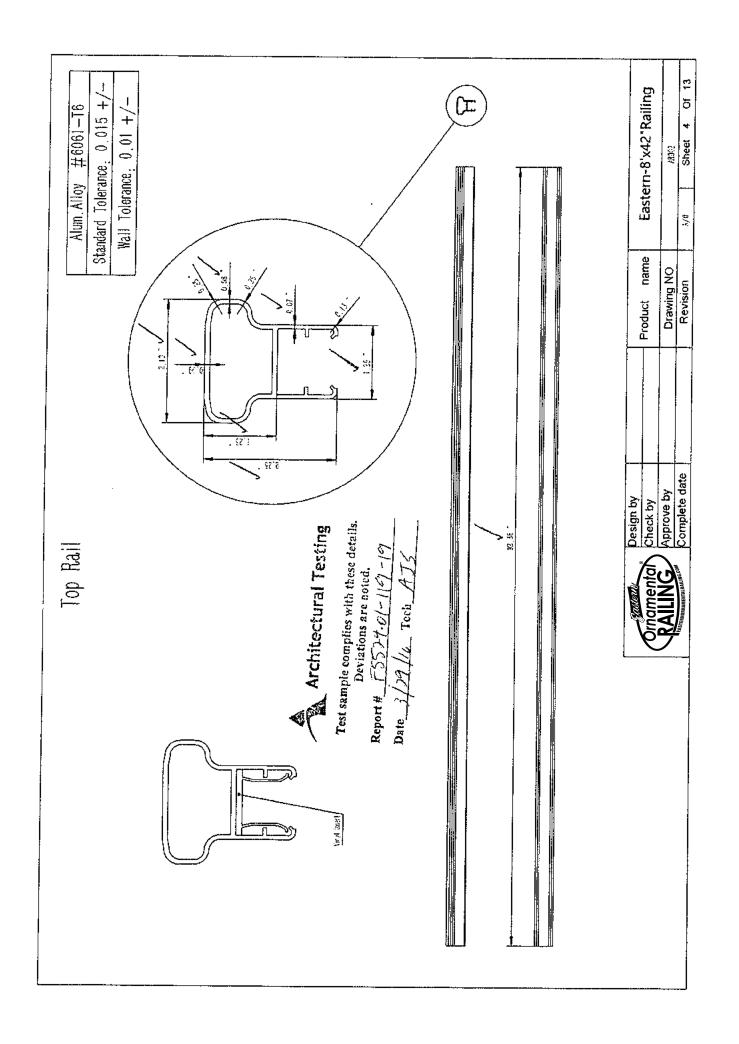
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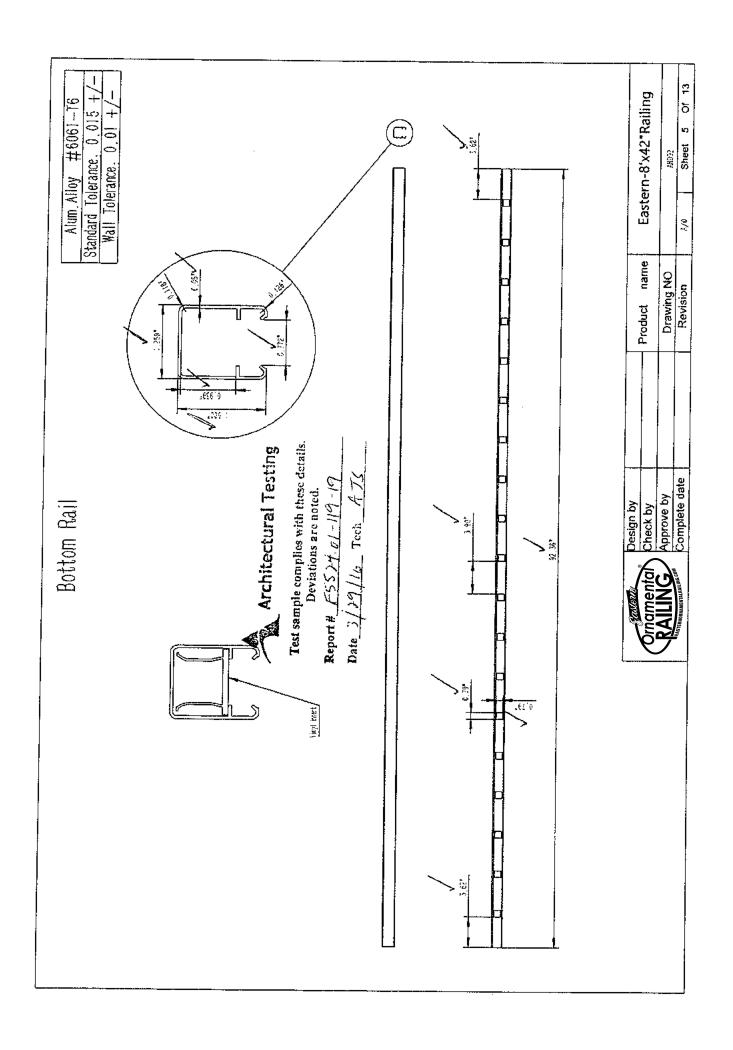
Date 3 /29 //6 Tech

	Parts List	ist	
S.N.	Item	QTY	Material
-	top rail 2125	-	Aluminum 6061-T6
7	bottom rail 12515	1	Aluminum 6061-T6
m	3/4" picket	19	Aluminum 6061-T6
4	Spacer	20	Aluminum 6061-T6
2	vinyl insert	-	vinyl
و	3" post	2	Aluminum 6061-T6
	base plate	2	#45 steel
8	0.4x2 3/4 Bolt (base plate to post)	8	carbon steel Level 8.8
6	3"Post trim	2	ADC-12
9	3" post cap	2	ADC-12
=	top bracket	2	ADC-12
12	bottom bracket	2	ADC-12
13	Screw (all brackets to post)	10	SS304
14	Screw (top bracket to rail)	4	SS304
15	bottom rail support	4	Aluminum 6061-T6







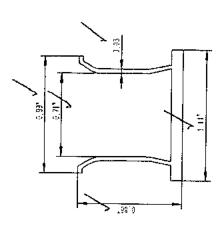


Alum. Alloy #6061—76
| Standard Tolerance: 0.015 +/| Wall Tolerance: 0.01 +/-7027.0 Test sample complies with these details. Deviations are noted. Architectural Testing Report # FSS34 01-119-19 37.03 3/4" Picket Date 3/39/14 Tech A75

	me Eastern-8'x42"Railing		3/0 Sheet 6 Of 13
	Product name	Drawing NO	Revision
			a
Design by	Check by	Approve by	Complete dat
aneros)	Ornamental	RAILING	EASTERNORNAMENTALRAILING.COM

Vinyl Insert

Material Vinyl	ndard Tolerance. 0.03 +/-	fall Tolerance, 0.02 +/-
	Standard	Wall



Architectural Testing

Test sample complies with these details.

Deviations are noted.

Report # F5534.01-119-19

Date 3/29/14 Teeb AJS

Design by	Check by	Approve by	Complete date
gasterin	Ornamental	RAILING!	EASTERNORNAMENTALRAILING.COM

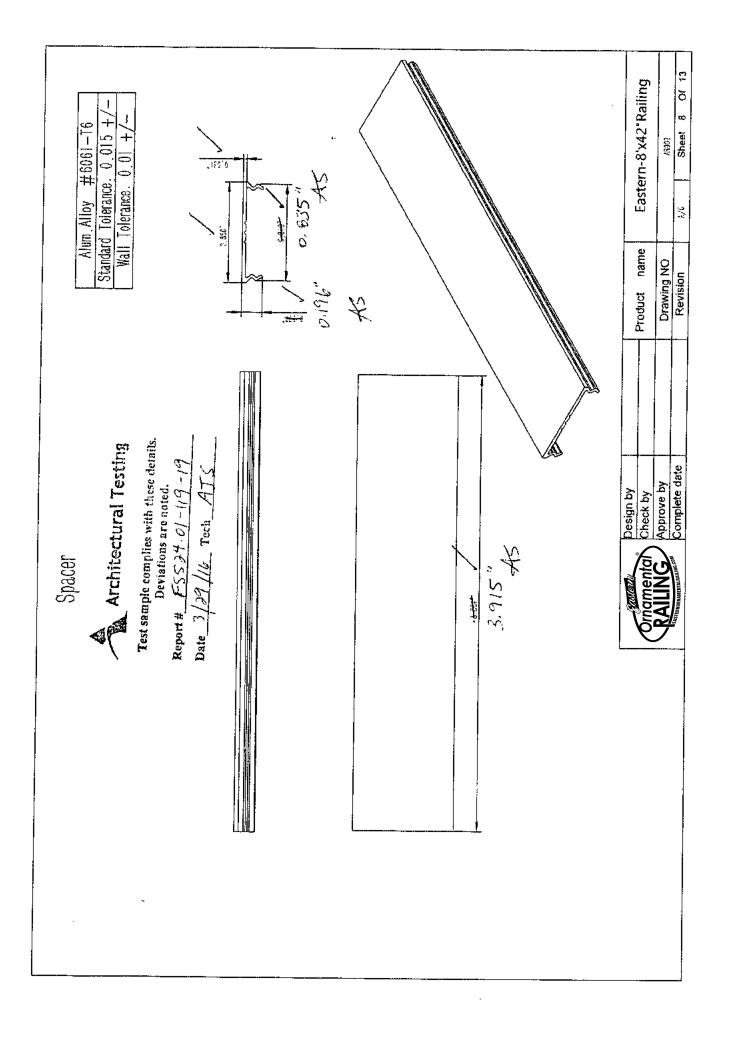
Maga Sheet 7 Of 13

9.7

Drawing NO Revision

Eastern-8'x42"Railing

Product name



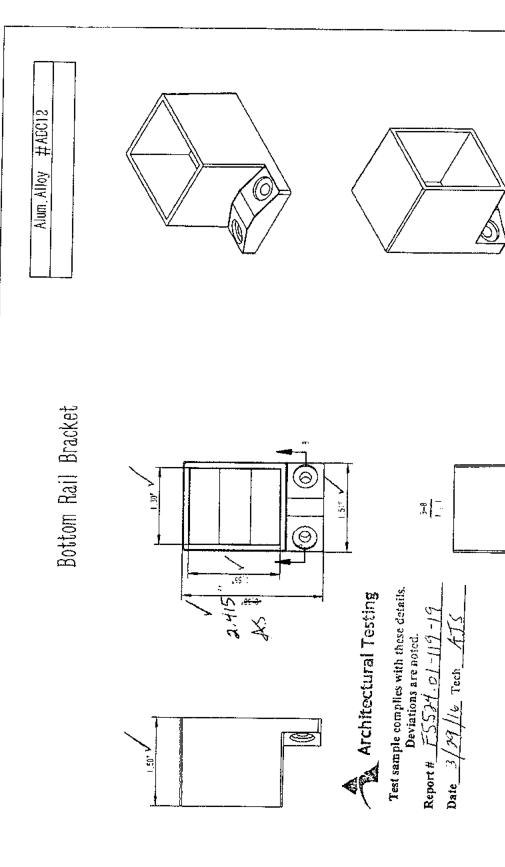
Alum. Alloy #ADC12 Architectural Testing 2,348. V \$ 115 \$1205, <

Top Rail Bracket

Test sample complies with these details. Deviations are noted. Report # F5524.01-119-10 Tech Date 3/29/14

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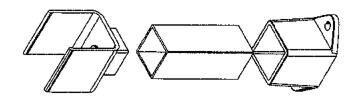
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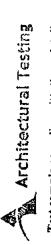




Bottom Rail Support

Alum Alloy #ADC12



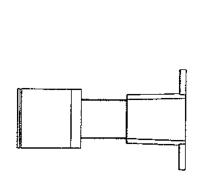


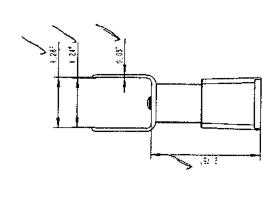
Test sample complies with these details. Deviations are noted.

Report # F5534.01-119-19 //6 Tech



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APPENDIX B

Photographs





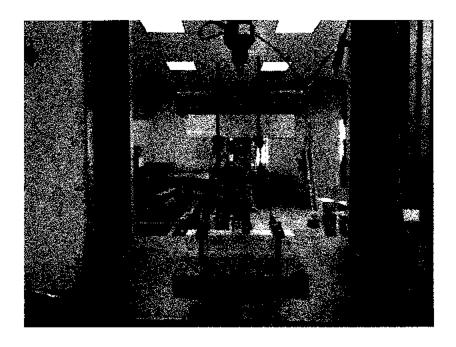


Photo No. 1
Assembly Fastener Test Setup



Photo No. 2 In-Fill Load Test at Center of Two Pickets







Photo No. 3
In-Fill Load Test at Bottom of Two Pickets



Photo No. 4
Concentrated Load Test at Mid-Span of Top Rail







Photo No. 5 Uniform Load Applied at 45 degrees



Photo No. 6
Concentrated Load at Ends of Top Rail (Brackets)





Photo No. 7
Stand-Alone Post Mount Testing